PROJECT: PROPOSED CONSTRUCTION OF BUILDING FOR

ABIRAMI PET INDUSTRIES PVT LTD,

GANGAIKONDAN,

TIRUNELVELI.

TITLE : SOIL INVESTIGATION REPORT

SUBMITTED TO : M/S. SAI SITE PLAN ENGINEERS,

NO: 80/41, SEMBIYAM REDHILLS ROAD,

KATHIRVEDU,

CHENNAI - 600 066.

SURVEY NO : 1599 pt, 1600 pt, 1601 pt, 1610 pt, 1611 pt, 1617 pt

SITE ADDRESS: **PLOT NO: 3B,**

SIPCOT INDUSTRIAL GROWTH CENTER,

GANGAIKONDAN,

TIRUNELVELI.

CONSULTANTS & CONTRACTORS:



HAMILTON MATERIAL TESTINGS PVT.LTD.

TESTING LAB FOR SOIL & BUILDING MATERIALS

REGD.NO: 2011/33/028/01317/DHDA/E

No.93-I/8, Ashok Nagar, 3rd Street,

Thoothukudi - 628 008.

Ph: 0461-2311367

Ref: /HMTPL Date: 04.10.18

SOIL TEST REPORT

The Soil samples collected from Two Bore Holes

(Standard penetration test samples and Disturbed Samples) at the

Proposed Construction of building for Abirami Pet Industries Pvt Ltd,
Gangaikondan, Tirunelveli.

The Soil Samples were analyzed in our laboratory and the test results are in Annexure.

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SOIL EXPLORATION:

Name of Project – Proposed Construction of building for Abirami Pet Industries Pvt. Ltd, Gangaikondan, Tirunelveli.

INTRODUCTION

Geotechnical Investigation for the "Proposed Construction of building for

Abirami Pet Industries Pvt. Ltd, Gangaikondan, Tirunelveli."

were carried out by us.

2. Scope of Work

Two boreholes were drilled through manually operated Auger drilling system on the proposed alignment of structures up to Hard Strata for preliminary identification of type and extent of soil layers. The Safe Bearing Capacity of layers at different depth is based on Standard Penetration test values which are conducted in the bore holes at regular intervals. Submitting geotechnical investigation report for the above project based on field and laboratory test results and recommendation of suitable foundation systems.

3. Field work

Field investigations were carried out on 26th September 2018 through manually operated Auger drilling system on the proposed site.

3.1 Drilling through soil medium

3.1.1 MANUALLY OPERATED AUGER BORING

Two boreholes were drilled through manually operated Auger drilling system.

The borehole walls, where weak, were supported by temporary steel casing.

3.1.2. STANDARD PENETRATION TEST

The test measures the penetration resistance of the split spoon sampler, when it is driven into the soil, at the bottom of a borehole in a standard manner. The N-value, which is the number of blow required to achieve 300mm penetration of the soil, indicates the relative density of sand or gravel, the consistency of other soil such as silts or clays and the strength of weak rocks. The test is described in IS 2131 - 1981.

The split spoon sampler is attached to stiff drill rod and lowered to the bottom of the bore hole. A standard blow consists of dropping a mass of 65kg free fall through 760 mm on to an anvil at the top of the rods and ensuing that this amount of dynamic energy is transferred to the sampler as much as possible.

The number of blows required to achieve each 150mm penetration is recorded for a fall penetration of 450mm. The initial 150mm penetration is referred to as seating drive and the blows required for this penetration are not considered as this zone is in disturbed soil. The next 300mm of penetration is referred to as the test drive and the number of blows required to achieve this fully is termed the penetration resistance or N-value. The SPT results are given in the annexure.

4. SAMPLING:

The disturbed samples were collected thro SPT sampler. Ground water table were observed one day after completion of investigation.

4. 1 LABORATORY TESTS

Following laboratory tests are conducted.

- a) Sieve analysis
- b) Specific gravity and Direct Shear Test.

5. Sub Soil Conditions

The soil conditions at this site found through Standard Penetration Test samples is uniform deposit of Medium Sand followed by Sandy Gravel Reddish Colour and the bore hole terminated at same layer.

6. Engineering Appraisal

Based on Laboratory results on the samples collected from investigation locations, the shear strength parameters such as C (Cohesive Strength) and Φ (angle of internal friction) can be correlated with SPT values.

7. Design Methodology

The Safe Bearing Capacity of the soil can be calculated on Shear failure Criteria and Settlement Criteria and any foundation system has to satisfy the above said criteria.

The former one depends on shear strength and the bearing capacity equation suggested by IS 6403 as follows.

Net ultimate Bearing Capacity is given by

C Nc Sc dc ic +
$$\gamma$$
 D (Nq - 1) Sq dq iq + 0.5 γ B N γ S γ d γ i γ W'

The Safe Bearing Capacity is the maximum gross intensity of loading that the soil can carry safely without failing in shear which is obtained by the Net ultimate Bearing Capacity divided by factor of safety (2.5 to 3.0)

The later one follows Settlement Criteria, that the intensity of the loading will cause within the permissible settlement is termed as allowable bearing pressure given by correcting the field SPT values

1. Correction due to Dialtancy (N'). (This correction is based on the assumption that critical void ratio occurs at approximately N = 15, and in fine grained cohessionless soils, the coefficient of permeability is low that the excess pore water pressure developed by the driving impedes the penetration of split spoon sampler, thus increasing the N value.)

$$N' = 15 + 0.5 (N_0 - 15)$$

2. Correction due to Overburden Pressure

$$N = 0.77 \text{ N' } \log_{10} (2000/\text{ p'}_{o})$$

p'_o effective over burden pressure should be greater than 25 kPaNet Allowable Bearing Pressure developed by Terzaghi and Peck (1948)

$$q_s = 35 (N_{cor} - 3) ((B + 0.3)/2B)^2 R_{w2} F_d$$

The Allowable Bearing Pressure of soil = Net allowable bearing Pressure of soil + p'o

8. Foundation recommendation:

It is recommended that the Shallow Foundations may be adopted in the Form of Isolated/ Combined Footing which is laid at the depth of 2.00m to 2.50m from the Existing Ground Level.

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Name of Project - Proposed Construction of building for

Abirami Pet Industries Pvt. Ltd, Gangaikondan, Tirunelveli.

Date of Commencement - 26.09.18 G.W. T.: Not Met

Date of Completion - 26.09.18 Bore Hole No. 1

Standard Popularies test date

			ï		Stand	test data	st data			
v G.L	ile		flaye	ch ted	ਮੁੱਤੇ N - Value depth of penetra				sity 3	
Depth below G.L. (R.L)	Soil Profile	Description of Soil	Thickness of layer (m)	Depth at which test is conducted	15cm	30cm	45cm	For 30cm	Relative density / consistency	
				1.00	5	8	10	18	Medium	
			4.50	2.00	12	16	20	36	Dense	
1.50		Medium Sand	1.50	3.00	24	27	50- 2cm	>50	V Dense	
	00000									
		Sandy Gravel Reddish	1.50							
3.00		End of Bore Hole								

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Date of Commencement - 26.09.18 Date of Completion - 26.09.18

Bore Hole No. 1

Soil Profile and depth in n Depth of Sampling IS Soil Classification Free Swell Index % Coarse Coarse Coarse Clay Blasticity Index Gravel Clay Clay Clay Cohesive Strength - T/m² Angle of Internal Friction Cobserved N Value Sift Cobserved N Value Safe Bearing Soil Classification Coarse Gravel Gravel Fine Clay Observed N Value Safe Bearing	neters					Atterberg Limits			Grain Size Distribution %						ear ength neters	Standard Penetration Test					
1.50 SP NP 6 4 49 38 3 0 0 31 2.00 36 26.	oth in m	npling	ication	% xəpı			X							С	ذ						
1.50 SP NP 6 4 49 38 3 0 0 31 2.00 36 26.	Soil Profile and dep	Depth of San IS Soil Classif	Free Swell I	Free Swell In	Free Swell Ir	Free Swell In	Free Swell In	Free Swell In	Liquid Limit Plastic Limit	Plasticity Inde	Gravel	Coarse	Coarse	Fine	Silt	Clay		Angle of Internal Friction	Depth of Testing M	Observed N Value	Safe Bearing Capacity T/ m²
																		15.77			
		1.50	SP		NP			6	4	49	38	3	0	0	31			26.95			
3.00 GW NP 53 5 23 15 4 0 0 34		3.00	GW		NP			53	5	23	15	4	0	0	34	3.00	>50	36.85			

NP - Non Plastic

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Name of Project - Proposed Construction of building for

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Date of Commencement - 26.09.18 G.W. T. : Not Met
Date of Completion - 26.09.18 Bore Hole No. 2

			ır	Standard Penetration test data									
w G.L	ile		flaye	ch ted	N – Value depth of pene				sity 3y				
Depth below G.L. (R.L.)	Soil Profile	Description of Soil	Thickness of (m)	Thickness of layer (m) Depth at which test is conducted		30cm	45cm	For 30cm	Relative density / consistency				
				1.00	3	5	11	16	Medium				
			4 80	2.00	9	14	17	31	Dense				
1.50		Medium Sand	1.50	3.00	19	>50	V Dense						
		Sandy Gravel Reddish	1.50										
3.00		End of Bore Hole											

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Date of Commencement -26.09.18 **Date of Completion** 26.09.18

Bore Hole No. 2

	Date of Completion -						20.09.10						DOTE HOLE NO. 2				-
eters					Atterberg Limits			Grain Size Distribution %						Shear Strength Parameters		Standard Penetration Test	
oth in m	Soil Profile and depth in meters Depth of Sampling IS Soil Classification	ication	% xəpı			X		Coarse Fine Grained Grained			С	ذ	"N" Value				
Soil Profile and dep		IS Soil Classification Free Swell Index %		Liquid Limit Plastic Limit		Gravel	Coarse	Medium	Fine	Silt	Clay	Cohesive Strength – T/m^2	Angle of Internal Friction	Depth of Testing M	Observed N Value	Safe Bearing Capacity T/ m ²	
	1.50	C.D.		NID			_	2	50	26	4	0	0	20	1.00	16	14.57
	1.50	SP		NP			5	3	52	36	4	0	0	29	2.00	31	23.87
	3.00	GW		NP			55	4	25	13	3	0	0	32	3.00	>50	36.85

NP - Non Plastic

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